# A Reasonable Approach to the Seismic Assessment of Water Supply Facilities



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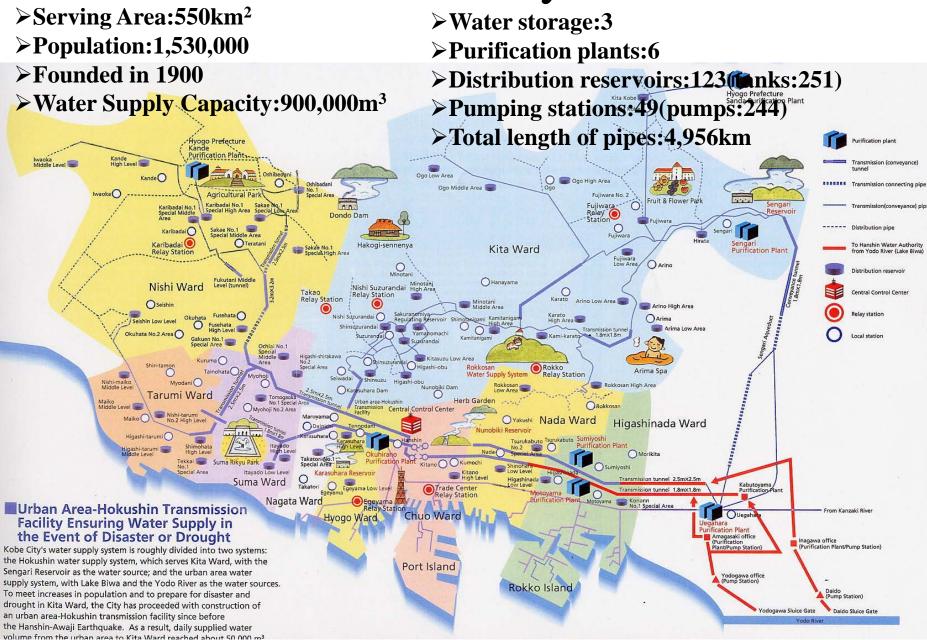
# Contents

- **1.** Background (Introduction)
- **2**. Seismic Assessment Procedures
- **3. Determining the Condition of Existing Facilities**
- **4.** Setting of Design Input Seismic Motions
- **5.** Analysis of Ground/Structual Properties
- 6. Setting a Two-Dimensional Model
- 7. Seismic Assessment
- 8. Conclusion

### **Location of Kobe City**



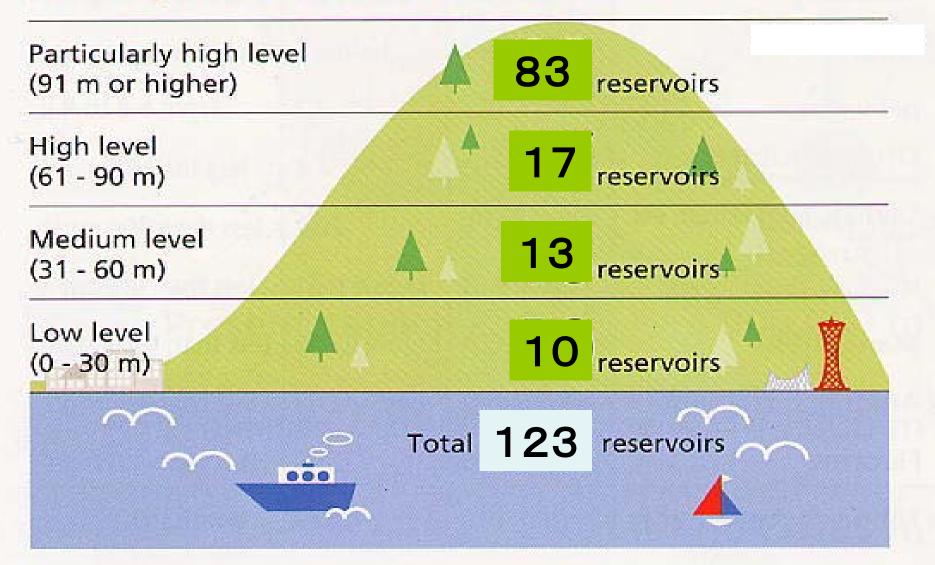
## Kobe Water System

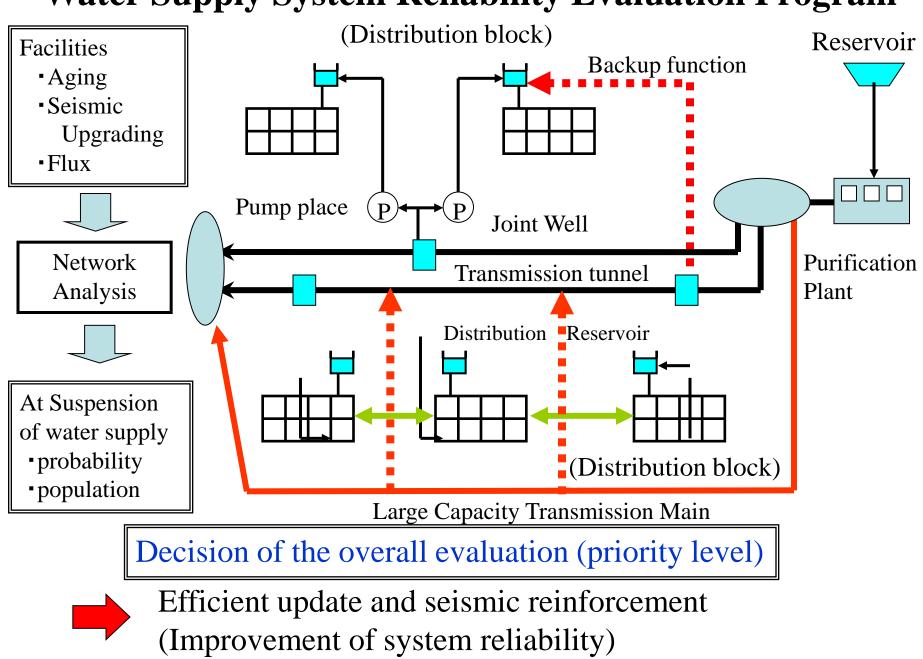


# Kobe Water System

Water Distribution System by Elevation

(Relationship between elevation and number of distribution reservoirs)





#### Water Supply System Reliability Evaluation Program

### SEISMIC ASSESSMENT PROCEDURES

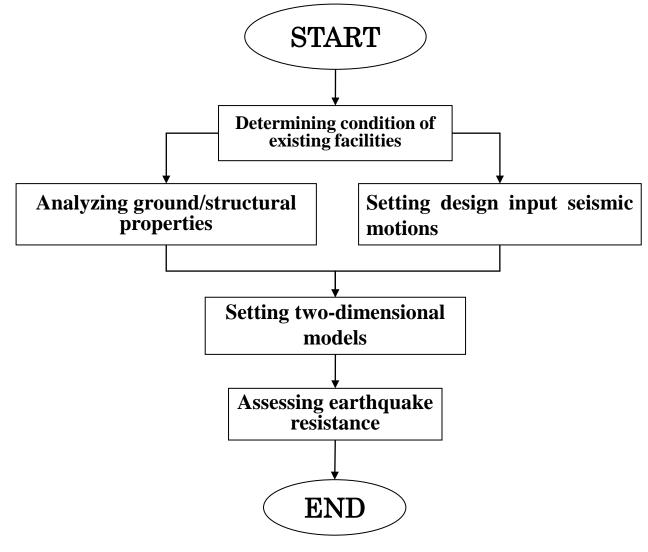
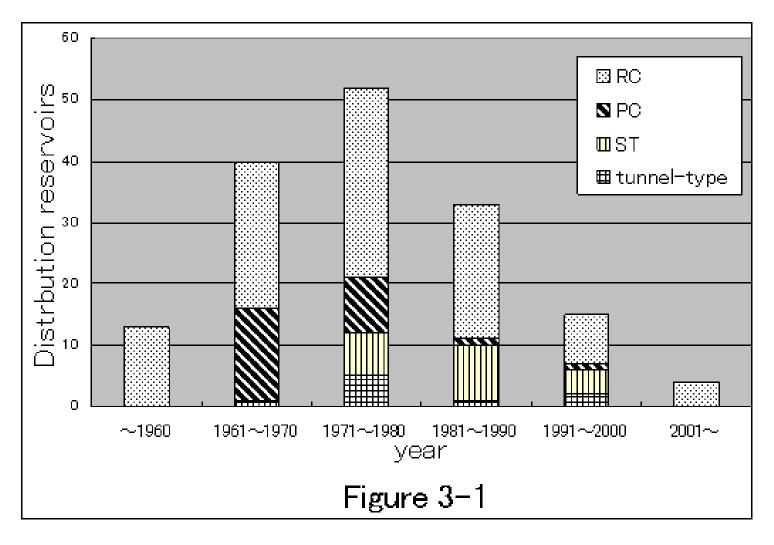


Figure 2-1

# DETERMINING THE CONDITION OF EXISTING FACILITIES



#### SETTING OF DESIGN INPUT SEISMIC MOTIONS

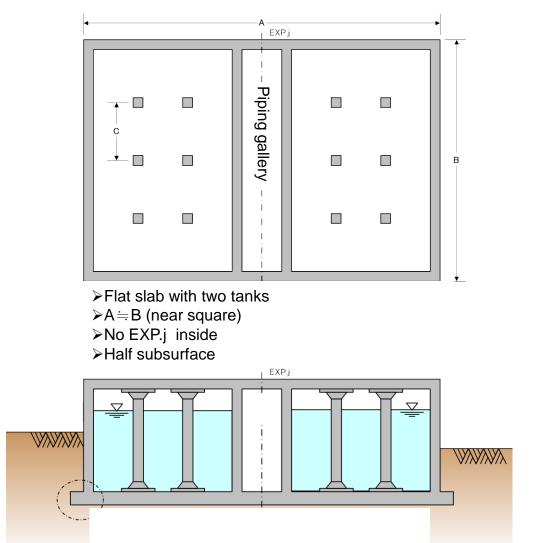
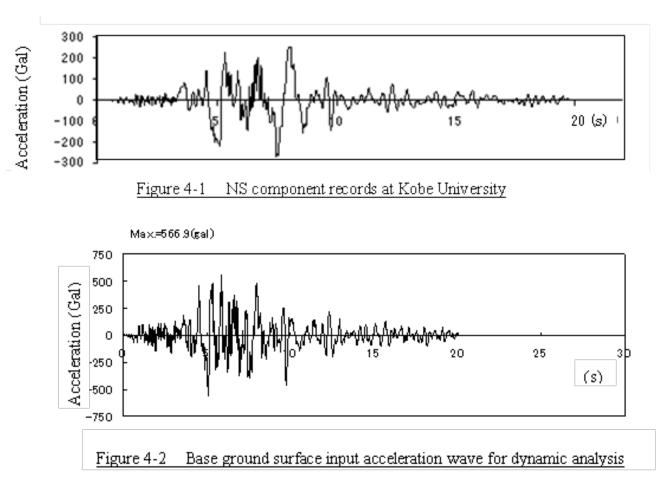


Figure Standard structure configuration of RC distribution reservoir

#### SETTING OF DESIGN INPUT SEISMIC MOTIONS

(1) Preparation of Base Ground Surface Input Waveforms



### (2) Dynamic Characteristics of Structures

[Analysis conditions]

- Foundation structure: Spread foundation
- Foundation ground: N value; 50
- Relaid soil: N value; 10 (sandy soil)
- Inside water level: High water level
- Elements: See figure at right

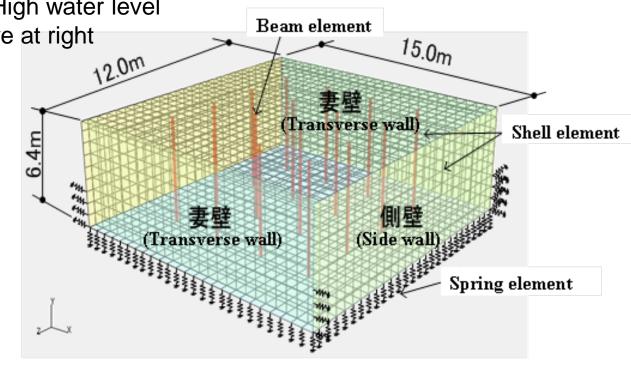


Figure 4-3

### (2) Dynamic Characteristics of Structures

	Deck slab	Upper slab	Side wall	Transverse wall	
Plate thickness t (mm)	550	300	550	550	
Young's modulus E (kN/m <sup>2</sup> )	2.500E+07	2.500E+07	2.500E+07	2.500E+07	
Poisson's ratio v	0.2	0.2	0.2	0.2	

 Table 4-1 Input physical properties of the building (shell element)

Table 4-2 Input physical properties of the center pillar (beam element)

Young's modulus E (kN/m <sup>2</sup> )	2.500E+07
Cross section A (m <sup>2</sup> )	0.25
Torsional constant J (m <sup>4</sup> )	8.788E-03
Geometrical moment of inertia $Iy = Iz (m^4)$	5.208E-03

Table 4-3 Input physical properties of the ground (spring element)

		Deck slab	Side wall	Transverse wall (end side)
Coefficient of	X direction	29404	17713	5111
subgrade reaction <sup>*2</sup>	Y direction	102912	5061	5111
(kN/m <sup>3</sup> )	Z direction	29404	5061	17888

#### (2) Dynamic Characteristics of Structures

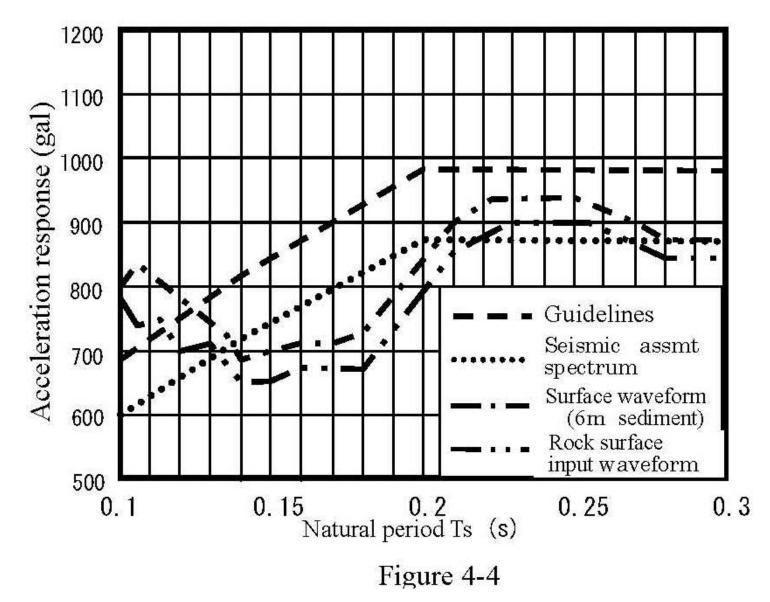
Table 4-4

		Natural period (s)		
		With fill soil	Without fill soil	
Vibration	X direction	0.1398	0.1272	
direction	Y direction	0.1576	0.1394	

Table 4-5

		Damping co	efficient (%)
		With fill soil	Without fill soil
Vibration	X direction	15.4	14.8
direction	Y direction	14.8	14.5

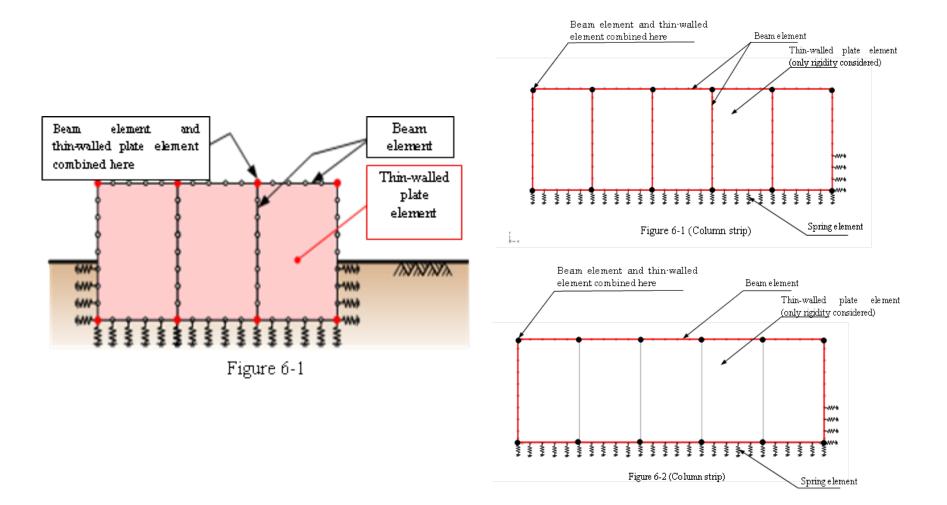
#### (3) Earthquake Response Analysis



## ANALYSIS OF GROUND/STRUCTURAL PROPERTIES

#### Table 5-1

Analysis cases	Parameters			
Case 1		Design water level		
Case 2	Water level	Operational water level		
Case 3		N=10		
Case 4		N=30		
Case 5	Fill soil	Yes / No		
Case 6	Unsymmetrical earth pressure	Yes / No		
Case 7	Embedment	Yes / No		
Case 8	Dimensions in planning	L/B=1.4		
Case 9	<b>X</b> Ratio of the space between transverse	L/B=2.2		
Case 10	(gable) walls (L) to the width of transverse(gable) walls (B)	L/B=3.0		



No.	Physical properties of th	Domorita	
INO.	Young's modulus E <sub>2</sub>	Plate thickness	Remarks
1	$E_{c}^{*}$	t <sub>eq</sub> **	
2	0.5 • E <sub>c</sub>	t <sub>eq</sub>	
3	0.2 • E <sub>c</sub>	t <sub>eq</sub>	
4	0.1 • E <sub>c</sub>	t <sub>eq</sub>	
5	0.05 • E <sub>c</sub>	t <sub>eq</sub>	
6	0.02 • E <sub>c</sub>	t <sub>eq</sub>	
7	0	0	No thin-walled plate element

Here, Ec (Young's modulus for the building's concrete) =  $2.5 \times 107$ kN/m2, and (equivalent plate thickness of the transverse wall) is calculated using the following equation:

$$t_{eq} = \frac{2 \times t_w}{B} = \frac{2 \times 0.55}{18.8} = 0.0585$$
 (m)

where (thickness of the transverse wall) = 0.55 (m), and (space between transverse walls, or depth of the structure) = 18.8 (m).

Table 6-2 Conditions	of the	structures
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	Deck slab	Upper slab	Sidewall	Transverse wall	Center pillar
Geometrics (mm)	Plate thickness t = 550	Plate thickness $t = 300$	Plate thickness t = 550	Plate thickness t = 550	500 × 500 ctc3650
Young's modulus E (kN/m <sup>2</sup> )	2.500E+07	2.500E+07	2.500E+07	2.500E+07	2.500E+07
Unit weight $\gamma$ (kN/m <sup>3</sup> )	24.5	24.5	24.5	24.5	24.5
Poisson's ratio $\nu$	0.2	0.2	0.2	0.2	0.2

Table 6-3 Coefficient of subgrade reaction  $(kN/m^3)$ 

		Lower part of deck slab	Sidewall	Transverse wall (end side)
	X direction	5758		
Under stationary load	Y direction	20153		
	Z direction	5758		
Under incremental	X direction	11516	17957	5181
load during	Y direction	40307	5131	5181
earthquake	Z direction	11516	5131	18135

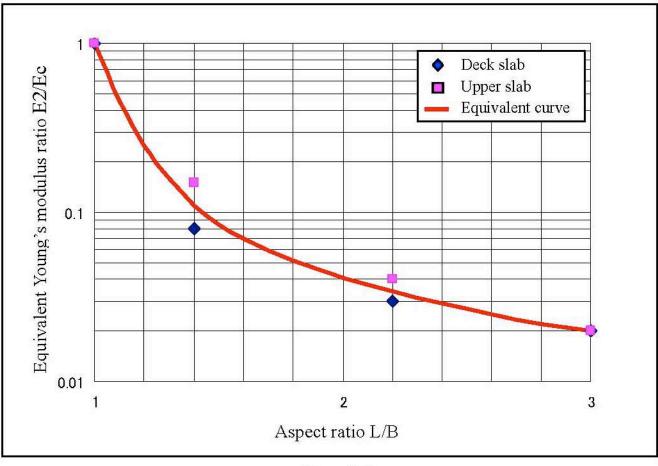


Figure 6-3

 Table
 Seismic assessment of RC distribution reservoir according to group(1)

G	roup	<b>Evaluation method</b>	Reason for grouping
Standard flat slab (above ground type) (pillar of two rows or more)	A-1-1-1( on the base ground surface, hard ground) (17) A-1-1-2(on the surface ground, hard ground) (19)	Comparative assessment based on result of analyzing representative facilities	Almost the same structural characteristic
	A-1-1-3(usual ground)	Detailed analysis of each facilities	Different seismic force condition in each facilities.
Standard flat slab (underground type)	A-1-2 (10)	Comparative assessment based on result of analyzing representative facilities	Almost the same structural characteristic
Standard flat slab (Small scale) (Pillar of one row or less)	A-2 (16)	Comparative assessment based on result of analyzing representative facilities	Almost the same structural characteristic
Wall construction without pillar and the similar	A-3-1 (wall-type) (4)	Comparative assessment based on result of analyzing representative facilities	Almost the same structural characteristic
	A-3-2 (similar to wall-type)	Technological evaluation of each facilities	Various structure characteristics & Data shortage

Group		<b>Evaluation method</b>	Reason for grouping
Pillar foundation structure with deck slab jointed	A-4	Technological evaluation of each facilities	Various structure characteristics & Data shortage
Similar structure to flat slab	A-5-1(Structure reinforced edge of deck or upper slab) A-5-2(Excluding the above-mentioned) A-8-1(Special condition)		
Composite structure	A-6	Undiagnosis	Special structure
Level 2 design structure	A-7	Undiagnosis	The latest design standard

**Technical assessment of earthquake** performance (for each group) Analysis results of representative facilities---OK Analysis results of representative facilities---NG Estimated bending moment Estimated bending moment generated

Figure7-1

#### Table Seismic Assessment Result

	Total	NG	ОК	Undiagnosis
RC	184	65(36%)	109(59%)	10(5%)
PC	29	21(72%)	8(28%)	0
ST	26	0	26(100%)	0
Tunnel-type	12	2(17%)	10(83%)	0
Total	251	88(35%)	153(61%)	10(4%)

# Thank you



Sunset with AKASHI Strait Bridge from Kobe Airport

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